# Harris Road-No Left Turn Sign

# Traffic Management Plan

Final Report

Prepared for: City of Canada Bay Council

Date: 27 March 2024

Ref: 305001438

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# Revision

Revision	Date	Comment	Prepared By	Approved By
A-Dr	27 March 2024	Draft	Sabal Sharma & Helen Aberra	Bayzid Khan
Final	14 May 2024	Final	Helen Aberra	Bayzid Khan

For and on behalf of

Stantec Australia Pty Ltd

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#### Acknowledgment of Country

In the spirit of reconciliation, Stantec acknowledges the Traditional Custodians of country throughout Australia and their connections to land, sea and community. We pay our respect to their Elders past and present, and extend that respect to all Aboriginal and Torres Strait Islander peoples.

#### Limitations

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# 1. Introduction

### 1.1 Overview

Stantec has been engaged by City of Canada Bay Council (Council) to prepare a Traffic Management Plan (TMP) for the proposed 'No Left Turn' for vehicles larger than 6m light vehicles at the Harris Road /Queens Road intersection.

Council have identified that large vehicles turning left from Harris Road onto Queens Road are mounting the existing kerb. Restriction for larger vehicles is required at the signalised intersection to ensure that vehicles turning left from Harris Road onto Queens Road do not mount the existing kerb.

Council's initial discussion with TfNSW indicated that this project will require a full TMP in accordance with the *TfNSW* 'Procedures for use in the preparation of a traffic management plan (*TMP*)'.

This TMP has been prepared in accordance with *Transport for NSW Procedures for Use in the Preparation of a Traffic Management Plan, Version 2.0, December 2001.* This includes an assessment of the impacts of the proposed no-left turn and required measures to ameliorate any impacts.

The proposed restricted movement is shown in Figure 1.1.



#### Figure 1.1: Subject Site and Its Environs

Base image source: Nearmap



# 2. Description of Proposal

The proposal is for 'No Left Turn' for vehicles larger than 6m light vehicles at the Harris Road /Queens Road intersection. Council have identified that large vehicles turning left from Harris Road onto Queens Road are mounting the existing kerb. Restriction for larger vehicles is required to at the signalised intersection is required to ensure that vehicles turning left from Harris Road onto Queens Road do not mount the existing kerb.

The proposed restricted movement is shown in Figure 1.1.

# 3. Identification and Assessment of Impacts of the Proposed Measures

### 3.1 Overview

Intersection capacity analysis has been conducted at key intersections near the site to assess the traffic implications of no left turn. The modelling scenarios are detailed in Table 3.1.

#### Table 3.1: Scenario analysis for modelling traffic impact

#	Scenario	Description
1	Existing Conditions	2024 traffic surveys
2	2024 With Left Turn Ban for vehicles larger than 6m light vehicles	Left turn ban

### 3.2 Existing Traffic Conditions

#### 3.2.1 Traffic Volumes

Traffic movement counts were completed on Tuesday 20/02/2024 for the following key intersections between 6:00am to 10:00am and 3:00pm to 7:00pm:

- Harris Road and Queens Road
- Harris Road and Kings Road

The traffic survey data also includes vehicle classification data based on Austroads Vehicle Classification System as shown indicatively in Figure 3.1.

It is to be noted that the Austroads Vehicle Classification System categorizes vehicles based on axle configuration or vehicle length. However, As shown in Figure 3.1, there is no specific class for vehicles up to 6 meters in length. As such, for the purposes for traffic modelling and intersection assessment, it is assumed that vehicles up to 5.5m long (Austroads Class 1) will be allowed to turn left from Harris Road onto Queens Road and vehicles greater than 5.5m in length (Austroads Class 2-12) will be restricted from doing such left turn.

Figure 3.2 and Figure 3.3 summarise the AM and PM peak hours respectively and are separated into two categories: Austroads Class 1 vehicles (vehicles less than 5.5m) and Austroads Class 2-12 vehicles (vehicles greater than 5.5m). The full traffic movement results are provided in Appendix A of this Traffic Management Plan.

Level 1	Leve	el 2	Level 3	4								
Length	Axles	and	venicie Type		AUSTROADS Classification							
(Indicative)		Groups	Typical Description	Class	Parameters	Typical Configuration						
Type	AA103	oroupa	Typical Description	Ciass	LIGHT VEHICLES							
Short			Short									
up to 5.5m		1 or 2	Sedan Wagon 4WD Litility	1	d(1) < 3 2m and axles = 2							
up to 0.011		1012	Light Van Bicycle Motorcycle etc	l .		200-1 200-1						
			Short - Towing	<u> </u>	aroups = 3							
	3 4 or 5	3	Trailer Caravan Boat etc	2	d(1) > 2.1m d(1) < 3.2m							
	3,4013		Taller, Garavall, Boat, etc	1 -	$d(1) \ge 2$ . IIII, $d(1) \le 3.2$ III, $d(2) \ge 2$ Im and a less = 3.4 or 5							
				L								
				<u> </u>								
	2	2	Two Axle Truck or Bus	3	d(1) > 3.2m and axles = 2							
Medium			1									
5.5m to 14.5m												
	3	2	Three Axle Truck or Bus	4	axles = 3 and groups = 2							
						and a second second second						
	> 3	2	Four Axle Truck	5	axles > 3 and groups = 2							
			Three Axle Articulated									
	3	3	Three axle articulated vehicle, or	6	d(1) > 3.2m, axles = 3							
			Rigid vehicle and trailer		and groups = 5							
							Four Axle Articulated					
	4	> 2	2 Four axle articulated vehicle, or	7	d(2) < 2.1m  or  d(1) < 2.1m  or  d(1) > 3.2m							
Long			Rigid vehicle and trailer		axies = 4 and groups $> 2$							
11.5m to 19.0m			Five Axle Articulated									
	5	> 2	> 2	Five axle articulated vehicle, or	8	d(2) < 2.1m  or  d(1) < 2.1m  or  d(1) > 3.2m						
			Rigid vehicle and trailer		axies = 5 and groups > 2							
			Six Axle Articulated									
	≥ 6	> 2	Six axle articulated vehicle, or	9	axies = 6 and groups > 2 or							
			Rigid vehicle and trailer		axies > 6 and groups = 3							
			B Double									
Medium	> 6	4	B Double, or	10	groups = 4 and axles > 6							
Combination			Heavy truck and trailer			<u>roximoto. 000 001 potimoto. 00 00</u>						
17.5m to 36.5m			Double Road Train		5							
	> 6	5 or 6	Double road train, or Medium articulated	11	groups = 5 or 6 and ayles > 6							
			vehicle and one dog trailer (M.A.D.)		and axies > 0	1011100 000 00 000 1011100 000 0 001						
Large			Triple Road Train									
Combination	> 6	> 6	Triple road train, or	12	groups > 6 and axles > 6							
Over 33.0m			Heavy truck and three trailers			<u> (0,111,9,0, 0,6, 0,6, 0,0, 0,0, 0,0, 0,0, 0,0,</u>						
	-											

#### **AUSTROADS Vehicle Classification System**

Group: Axle group, where adjacent axles are less than 2.1m apart Groups: Number of axle groups Axles: Number of axles (maximum axle spacing of 10.0m)

d(1): Distance between first and second axle d(2): Distance between second and third axle

Source: Austroads 1994









#### 3.2.2 Intersection Performance

The commonly used measure of intersection performance, as defined by Transport for NSW, is vehicle delay. SIDRA Intersection determines the average delay that vehicles encounter and provides a measure of the level of service. A Level of Service of D or better is generally considered acceptable operation.

Table 3.2 shows the criteria that SIDRA Intersection adopts in assessing the level of service.

Level of Service	Average delay per vehicle (secs/ veh)	Average delay per vehicle (secs/ veh) Traffic signals, roundabout			
А	Less than 14	Good operation	Good operation		
В	15 to 28	Good with acceptable delays and spare capacity	Acceptable delays and spare capacity		
С	29 to 42	Satisfactory	Satisfactory, but accident study required		
D	43 to 56	Near capacity	Near capacity, accident study required		
E	57 to 70	At capacity, at signals incidents will cause excessive delays	At capacity, requires other control mode		
F	Greater than 70	Extra capacity required	Extreme delay, major treatment required		

Table 3.2: SIDRA Intersection level of service criteria

SCATS signal information has been obtained from Transport for NSW to assist with coding the signalised intersection. SIDRA calculated cycle time/ phase times were calibrated to align with the SCATS cycle time/ phase times. A site visit was conducted, and the models were checked to ensure that calculated queues were similar to what was observed during the site visit. Pedestrian protections were coded at the signalised intersection of Harris Road and Queens Road intersection in accordance with existing conditions.

Table 3.3 presents a summary of the existing operation of the key intersections during both peak periods.

#### Table 3.3: Existing operation condition

Intersection	Control	Peak Hour	Degree of Saturation	Average delay (seconds)	Average queue (metres)	Level of Service
Harris Road and Queens Road	Signalised	AM	0.67	31	96	С
		PM	0.81	33	125	С
Harris Road and Kings Road	is Road and Priority		0.22	17	2	С
	-	PM	0.16	15	2	В

All intersections assessed currently operate satisfactorily, with a Level of Service C or better and spare capacity during both AM and PM peak hours.

### 3.3 Traffic Redistribution

The 'No Left Turn' from Harris Road (southern approach) into Queens Road results in a re-assignment of the traffic. The vehicles (larger than 5.5m light vehicles) taking the left turn from Harris Road onto Queens Road are summarised in Table 3.4.

#### Table 3.4: Harris Road left turn volumes (Class 2-12)

Peak Hour Traffic	Austroads User Class 3	Austroads User Class 4
Harris Road Left onto Queens Road AM	1	1
Harris Road Left onto Queens Road PM	0	0

Table 3.4 shows that there are only two vehicles (more than 5.5m) turning left onto Queens Road during the AM. This includes one User Class 3 vehicle and one User Class 4. There were no vehicles greater than 5.5m turning left during the PM peak.

It is assumed that there will likely be two diversion routes for heavy vehicles based on the direction of travel, namely Route 1 and Route 2 as shown in Figure 3.4. Route 1 involves both eastbound and westbound heavy vehicles travelling



north via Harris Road and turning left from Kings Road. However, it is also assumed that some westbound heavy vehicles may also turn right into Great Northern Road and turn left onto Queens Road as shown in Route 2.





Base image source: Nearmap

### 3.4 Traffic Impact Analysis

The performance of the intersections for the No Left Turn' for vehicles larger than 6m light vehicles from Harris Road (south approach) into Queens Road is summarised in Table 3.5.

As shown in Table 3.4, only two vehicles larger than 5.5m is currently turning left from Harris Road onto Queens Road during AM peak hour. No heavy vehicle is turning left currently from Harris Road onto Queens Road in the PM peak. The intersection traffic modelling has been updated by adjusting the intersection turn flows for heavy vehicles in the AM peak hour only based on the two diversion routes discussed above. No update/adjustment is made to the PM peak hour traffic model, hence no change to the PM peak model results for the two diversion route options.

Intersection	Scenario	Control	Peak Hour	Degree of Saturation	Average delay (seconds)	Average queue (metres)	Level of Service
	Evipting		AM	0.67	31	96	С
	Existing		PM	0.81	33	125	С
Harris Road	Diversion	Signalized	AM	0.67	31	96	С
Road	route 1	Signalised	PM	0.81	33	125	С
	Diversion		AM	0.67	31	97	С
	route 2		PM	0.81	33	125	С
	Evipting		AM	0.22	19	2	С
	Existing		PM	0.17	17	2	В
Harris Road	Diversion	<b>Briority</b>	AM	0.22	19	2	В
Road	route 1	Priority	PM	0.16	15	2	В
	Diversion		AM	0.22	19	2	В
	route 2		PM	0.16	15	2	В

Table 3.5: Intersection Performance Results (left turn ban)



Table 3.5 demonstrates that all intersections will continue to operate well at a Level of Service of C or better with spare capacity during both AM and PM peak hours in all scenarios with no material change to the average delays. As such, the two heavy vehicles diverted in the AM peak hour will make no material difference to the operation of the modelled network and the surrounding road network.

Overall, the above modelling indicates that no left turn for heavy vehicles from Harris Road onto Queens Road can be supported from a traffic impact perspective.

# 4. Measures to Ameliorate the Impacts of Reassigned Traffic

The two heavy vehicles diverted in the AM peak hour will make no material difference to surrounding road network. Furthermore, the road geometry of the rerouted route along Harris Road-Kings Road-Queens Road and Paramatta Road -Great North Road- Queens Road can accommodate the rerouted vehicles. Hence, there are no measures required to ameliorate re-assigned traffic due to no adverse impact being identified as a result of the proposal.

### 5. Assessment of Public Transport Services Affected

There are no public transport services that are affected by the proposal. The public transport services travel through along Harris Road or take a southbound right turn from the northern approach of Harris Road. There are no known public or school bus services taking a left turn from the southern approach of Harris Road onto Queens Road.

The existing bus services travelling along Harris Road along with the route are summarised in Table 5.1.

Bus	Route	Movement
406	Hurlstone Park to Five Dock	Harris Road Through
415	Campsie to Chiswick	Harris Road Through
530	Chatswood to Burwood	Harris Road Through
570s	Rosebank College to Five Dock Shops	Harris Road Through
571s	Rosebank College to Five Dock Shops	Harris Road Through
577s	Rodd Point to St Patricks Strathfield	Harris Road Through
578s	St Patricks Strathfield to Five Dock Shops	Harris Road Through
598s	Earlwood to Hunters High	Harris Road Through
679s	Domremy College to Bexley Road and William Street, Kingsgrove	Harris Road Through
568s	Rosebank College to Strathfield Station	Southbound Right onto Queens Road
573s	Rosebank College to Concord Hospital	Southbound Right onto Queens Road
574s	Rosebank College to Homebush Station	Southbound Right onto Queens Road
575s	Homebush Station to Rosebank College	Southbound Right onto Queens Road
656s	Concord High School to Five Dock Shops	Southbound Right onto Queens Road
665s	Domremy College to Sydney Olympic Park	Southbound Right onto Queens Road
674s	Domremy College to Concord West Station	Southbound Right onto Queens Road
677s	Domremy College to Concord Shops	Southbound Right onto Queens Road

#### Table 5.1: Existing public transport

# 6. Details of Provision Made for Emergency Vehicles, Heavy Vehicles, Cyclists and Pedestrians

No special provisions for emergency vehicles will be required. Emergency vehicles more than 6m long will have alternative routes available along the road network including King Street. There are no additional provisions provided for heavy vehicles as alternate routes remain available. Cyclist and pedestrian access will be maintained.

# 7. Assessment of Effect on Existing and Future Developments with Transport Implications in the Vicinity of the Proposed Measure

No other known developments of significance are expected to be impacted by the proposal.

# 8. Assessment of Effect of Proposed Measures on Traffic Movements in Adjoining Council Areas

The two heavy vehicles diverted in the AM peak hour proposal will not affect traffic movements in adjoining council areas.

# 9. Public Consultation

The council will be undertaking community consultation on the proposal separately.

# Appendix A. Traffic Volumes





PM Peak hour





### Redirected AM Peak hour scenario



Appendix B. SIDRA Modelling Results



# Site: 0433 [Queens Rd/ Harris Rd\_AM\_Existing (Site Folder: General)]

Output produced by SIDRA INTERSECTION Version: 9.0.1.9664

Reprocess the Site in this Version to see the selected Movement Class results. All results may be affected by reprocessing compared with Version 9.0 results.

New Site

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 110 seconds (Site User-Given Cycle Time)

Vehicle Movement Performance															
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	Aver.	Back Of	Prop.	Eff.	Aver.	Aver.
ID		Class	FI	lows	FI	ows	Satn	Delay	Service	Qı	leue	Que	Stop	No. of	Speed
			[ Total	HV ] %	[ Total	HV ] %	v/c	sec		[ Veh. veh	Dist ] m		Rate	Cycles	km/h
South	: Harr	is Road	VON/IT	70	VOII/II	70	v/0	000		VOIT					1X11/11
4	L2	All MCs	149	1.4	149	1.4	0.268	34.2	LOS C	3.8	26.8	0.80	0.74	0.80	29.2
5	T1	All MCs	313	6.7	313	6.7	*0.669	36.6	LOS C	10.0	73.8	0.93	0.81	0.93	23.9
6	R2	All MCs	35	3.0	35	3.0	0.669	42.2	LOS C	10.0	73.8	0.93	0.81	0.93	32.1
Appro	ach		497	4.9	497	4.9	0.669	36.2	LOS C	10.0	73.8	0.89	0.79	0.89	26.5
East:	Quee	ns Road													
7	L2	All MCs	94	1.1	94	1.1	0.666	30.3	LOS C	13.5	95.9	0.86	0.78	0.86	30.8
8	T1	All MCs	400	1.6	400	1.6	*0.666	27.0	LOS B	13.5	95.9	0.86	0.78	0.86	30.7
9	R2	All MCs	24	0.0	24	0.0	0.666	30.5	LOS C	13.5	95.9	0.86	0.78	0.86	26.2
Appro	ach		518	1.4	518	1.4	0.666	27.8	LOS B	13.5	95.9	0.86	0.78	0.86	30.6
North:	Harri	s Road													
10	L2	All MCs	69	3.0	69	3.0	0.548	37.5	LOS C	8.8	63.3	0.89	0.77	0.89	24.2
11	T1	All MCs	469	2.7	469	2.7	0.548	35.2	LOS C	8.8	63.3	0.89	0.77	0.89	23.8
12	R2	All MCs	46	0.0	46	0.0	0.548	39.9	LOS C	7.4	52.9	0.90	0.77	0.90	23.5
Appro	ach		585	2.5	585	2.5	0.548	35.8	LOS C	8.8	63.3	0.89	0.77	0.89	23.8
West:	Quee	ens Road													
1	L2	All MCs	153	4.1	153	4.1	0.193	24.4	LOS B	3.1	22.1	0.65	0.70	0.65	27.7
2	T1	All MCs	465	2.5	465	2.5	0.559	24.7	LOS B	11.3	80.3	0.80	0.71	0.80	31.5
Appro	ach		618	2.9	618	2.9	0.559	24.6	LOS B	11.3	80.3	0.76	0.70	0.76	30.8
All Ve	hicles		2218	2.9	2218	2.9	0.669	30.9	LOS C	13.5	95.9	0.85	0.76	0.85	28.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

\* Critical Movement (Signal Timing)

Pedestrian Movement Performance											
Mov	Input	Dem.	Aver.	Level of	AVERAGE	E BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID Crossing	Vol.	Flow	Delay	Service	QU [ Ped	EUE Dist ]	Que	Stop Rate	Time	Dist. S	Speed
	ped/h	ped/h	sec		ped	m			sec	m	m/sec
South: Harris	Road										

P2 Full	50	53	49.3	LOS E	0.2	0.2	0.95	0.95	214.5	214.8	1.00
East: Queens	Road										
P3 Full	50	53	49.3	LOS E	0.2	0.2	0.95	0.95	211.8	211.3	1.00
North: Harris F	Road										
P4 Full	50	53	49.3	LOS E	0.2	0.2	0.95	0.95	214.7	215.0	1.00
West: Queens	Road										
P1 Full	50	53	49.3	LOS E	0.2	0.2	0.95	0.95	213.0	212.9	1.00
All Pedestrians	200	211	49.3	LOS E	0.2	0.2	0.95	0.95	213.5	213.5	1.00

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA INTERSECTION 9.1 | Copyright © 2000-2022 Akcelik and Associates Pty Ltd | sidrasolutions.com Organisation: STANTEC NEW ZEALAND | Licence: NETWORK / Enterprise Level 5 | Processed: Tuesday, May 14, 2024 10:33:35 AM Project: \\au2012-ntap01\_cifs02\shared\_projects\305001438\Des An\Traffic\240508\_harris\_road\_pedesterian\_fence.sip9

# Site: 0433 [Queens Rd/ Harris Rd\_PM\_Existing (Site Folder: General)]

Output produced by SIDRA INTERSECTION Version: 9.0.1.9664

Reprocess the Site in this Version to see the selected Movement Class results. All results may be affected by reprocessing compared with Version 9.0 results.

New Site

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 105 seconds (Site User-Given Cycle Time)

Vehic	le M	ovement	t Perfo	rma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	Aver.	Back Of	Prop.	Eff.	Aver.	Aver.
ID		Class	FI	lows	FI	ows	Satn	Delay	Service	QL	leue	Que	Stop	No. of	Speed
			[ lotal veh/h	HV J %	[ lotal   veh/h	HV J %	v/c	sec		[ Veh. veh	Dist J m		Rate	Cycles	km/h
South	: Harr	is Road													
4	L2	All MCs	204	0.0	204	0.0	0.328	31.6	LOS C	4.8	33.6	0.79	0.76	0.79	29.8
5	T1	All MCs	409	2.1	409	2.1	*0.796	38.1	LOS C	13.7	97.2	0.97	0.91	1.05	23.4
6	R2	All MCs	41	0.0	41	0.0	0.796	43.7	LOS D	13.7	97.2	0.97	0.91	1.05	31.6
Appro	ach		655	1.3	655	1.3	0.796	36.4	LOS C	13.7	97.2	0.91	0.86	0.97	26.4
East:	Quee	ns Road													
7	L2	All MCs	71	0.0	71	0.0	0.812	36.8	LOS C	17.9	125.4	0.96	0.91	1.02	29.3
8	T1	All MCs	515	0.4	515	0.4	*0.812	33.3	LOS C	17.9	125.4	0.95	0.90	1.02	29.2
9	R2	All MCs	20	0.0	20	0.0	0.812	36.9	LOS C	17.9	125.4	0.96	0.91	1.02	24.4
Appro	ach		605	0.3	605	0.3	0.812	33.9	LOS C	17.9	125.4	0.95	0.90	1.02	29.1
North:	Harri	s Road													
10	L2	All MCs	36	0.0	36	0.0	0.487	33.4	LOS C	7.8	55.7	0.85	0.73	0.85	25.5
11	T1	All MCs	425	2.5	425	2.5	0.487	32.2	LOS C	7.8	55.7	0.87	0.74	0.87	24.7
12	R2	All MCs	56	0.0	56	0.0	0.487	39.6	LOS C	5.5	39.4	0.90	0.76	0.90	23.5
Appro	ach		517	2.0	517	2.0	0.487	33.1	LOS C	7.8	55.7	0.87	0.75	0.87	24.6
West:	Quee	ns Road													
1	L2	All MCs	86	0.0	86	0.0	0.116	25.3	LOS B	1.7	11.8	0.66	0.68	0.66	27.4
2	T1	All MCs	416	0.3	416	0.3	0.540	25.9	LOS B	10.0	69.6	0.82	0.72	0.82	31.2
Appro	ach		502	0.2	502	0.2	0.540	25.8	LOS B	10.0	69.6	0.79	0.71	0.79	30.7
All Ve	hicles		2279	1.0	2279	1.0	0.812	32.6	LOS C	17.9	125.4	0.89	0.81	0.92	27.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

\* Critical Movement (Signal Timing)

Pedestrian I	Novem	ent Perf	ormano	e:						
Mov	Input	Dem.	Aver.	Level of	AVERAGE	E BACK OF	Prop.	Eff.	Travel	Travel Aver.
ID Crossing	Vol.	Flow	Delay	Service	QU [ Ped	EUE Dist 1	Que	Stop Rate	Time	Dist. Speed
	ped/h	ped/h	sec		ped	m			sec	m m/sec
South: Harris	Road									

P2 Full	50	53	46.8	LOS E	0.1	0.1	0.94	0.94	212.0	214.8	1.01
East: Queens	Road										
P3 Full	50	53	46.8	LOS E	0.1	0.1	0.94	0.94	209.3	211.3	1.01
North: Harris F	Road										
P4 Full	50	53	46.8	LOS E	0.1	0.1	0.94	0.94	212.2	215.0	1.01
West: Queens	Road										
P1 Full	50	53	46.8	LOS E	0.1	0.1	0.94	0.94	210.5	212.9	1.01
All Pedestrians	200	211	46.8	LOS E	0.1	0.1	0.94	0.94	211.0	213.5	1.01

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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#### V Site: 101 [Harris Road/ Kings Road\_AM\_Existing (Site Folder: General)]

Output produced by SIDRA INTERSECTION Version: 9.0.1.9664

Reprocess the Site in this Version to see the selected Movement Class results. All results may be affected by reprocessing compared with Version 9.0 results.

New Site Site Category: (None) Give-Way (Two-Way)

#### Vehicle Movement Performance

				inia											
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	Aver.	Back Of	Prop.	Eff.	Aver.	Aver.
ID		Class	FI	lows	FI	ows	Satn	Delay	Service	QL	leue	Que	Stop	No. of	Speed
			[ Total	HV ]	[ Total	HV ]				[Veh.	Dist ]		Rate	Cycles	
			veh/h	%	veh/h	%	V/C	sec		veh	m				km/h
South	: Harr	is Road													
1	L2	All MCs	49	2.1	49	2.1	0.260	4.7	LOS A	0.0	0.0	0.00	0.06	0.00	56.2
2	T1	All MCs	438	6.0	438	6.0	0.260	0.0	LOS A	0.0	0.0	0.00	0.06	0.00	59.0
Appro	ach		487	5.6	487	5.6	0.260	0.5	NA	0.0	0.0	0.00	0.06	0.00	58.7
East:	Kings	Road													
4	L2	All MCs	28	0.0	28	0.0	0.050	7.7	LOS A	0.1	0.5	0.54	0.72	0.54	45.3
5	T1	All MCs	6	0.0	6	0.0	0.050	14.0	LOS A	0.1	0.5	0.54	0.72	0.54	51.3
Appro	ach		35	0.0	35	0.0	0.050	8.8	LOS A	0.1	0.5	0.54	0.72	0.54	46.8
North	: Harri	s Road													
7	L2	All MCs	22	0.0	22	0.0	0.375	8.5	LOS A	0.6	4.3	0.25	0.11	0.30	56.4
8	T1	All MCs	537	2.7	537	2.7	0.375	0.9	LOS A	0.6	4.3	0.25	0.11	0.30	56.4
9	R2	All MCs	88	0.0	88	0.0	0.375	8.7	LOS A	0.6	4.3	0.25	0.11	0.30	55.8
Appro	ach		647	2.3	647	2.3	0.375	2.2	NA	0.6	4.3	0.25	0.11	0.30	56.3
West:	Kings	Road													
10	L2	All MCs	89	0.0	89	0.0	0.222	7.4	LOS A	0.3	2.2	0.58	0.77	0.58	50.1
11	T1	All MCs	18	0.0	18	0.0	0.222	14.9	LOS B	0.3	2.2	0.58	0.77	0.58	50.2
12	R2	All MCs	21	0.0	21	0.0	0.222	19.3	LOS B	0.3	2.2	0.58	0.77	0.58	43.5
Appro	ach		128	0.0	128	0.0	0.222	10.4	LOS A	0.3	2.2	0.58	0.77	0.58	49.3
All Ve	hicles		1298	3.2	1298	3.2	0.375	2.5	NA	0.6	4.3	0.20	0.17	0.22	55.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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#### V Site: 101 [Harris Road/ Kings Road\_PM\_Exsting (Site Folder: General)]

Output produced by SIDRA INTERSECTION Version: 9.0.1.9664

Reprocess the Site in this Version to see the selected Movement Class results. All results may be affected by reprocessing compared with Version 9.0 results.

New Site Site Category: (None) Give-Way (Two-Way)

#### Vehicle Movement Performance

Mov ID	Turn	Mov Class	Dem Fl [ Total veh/h	nand lows HV ] %	Ar Fl [ Total veh/h	rival lows HV ] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	Aver. Q [ Veh. veh	Back Of ueue Dist ] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Harr	is Road													
1	L2	All MCs	32	0.0	32	0.0	0.269	4.8	LOS A	0.0	0.0	0.00	0.04	0.00	56.6
2	T1	All MCs	485	1.7	485	1.7	0.269	0.0	LOS A	0.0	0.0	0.00	0.04	0.00	59.3
Appro	ach		517	1.6	517	1.6	0.269	0.3	NA	0.0	0.0	0.00	0.04	0.00	59.2
East:	Kings	Road													
4	L2	All MCs	23	0.0	23	0.0	0.055	7.3	LOS A	0.1	0.5	0.54	0.72	0.54	45.1
5	T1	All MCs	13	0.0	13	0.0	0.055	12.1	LOS A	0.1	0.5	0.54	0.72	0.54	51.2
Appro	ach		36	0.0	36	0.0	0.055	9.0	LOS A	0.1	0.5	0.54	0.72	0.54	47.8
North	Harri	s Road													
7	L2	All MCs	15	0.0	15	0.0	0.304	8.4	LOS A	0.3	2.4	0.18	0.08	0.20	56.9
8	T1	All MCs	469	2.2	469	2.2	0.304	0.6	LOS A	0.3	2.4	0.18	0.08	0.20	57.4
9	R2	All MCs	52	4.1	52	4.1	0.304	8.7	LOS A	0.3	2.4	0.18	0.08	0.20	56.1
Appro	ach		536	2.4	536	2.4	0.304	1.6	NA	0.3	2.4	0.18	0.08	0.20	57.2
West:	Kings	Road													
10	L2	All MCs	68	0.0	68	0.0	0.170	7.5	LOS A	0.2	1.7	0.58	0.77	0.58	50.2
11	T1	All MCs	9	0.0	9	0.0	0.170	12.7	LOS A	0.2	1.7	0.58	0.77	0.58	50.4
12	R2	All MCs	23	0.0	23	0.0	0.170	16.7	LOS B	0.2	1.7	0.58	0.77	0.58	43.7
Appro	ach		101	0.0	101	0.0	0.170	10.1	LOS A	0.2	1.7	0.58	0.77	0.58	49.2
All Ve	hicles		1189	1.8	1189	1.8	0.304	2.0	NA	0.3	2.4	0.15	0.14	0.15	56.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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# Site: 0433 [Queens Rd/ Harris Rd\_AM\_Existing - scenario 1 (Site Folder: Left turn ban)]

Output produced by SIDRA INTERSECTION Version: 9.0.1.9664

Reprocess the Site in this Version to see the selected Movement Class results. All results may be affected by reprocessing compared with Version 9.0 results.

New Site

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 110 seconds (Site User-Given Cycle Time)

Vehic	le M	ovement	t Perfo	rma	nce										
Mov	Turn	Mov	Dem	nand	Ar	rival	Deg.	Aver.	Level of	Aver.	Back Of	Prop.	Eff.	Aver.	Aver.
ID		Class	FI	lows	FI	ows	Satn	Delay	Service	Q	Jeue	Que	Stop	No. of	Speed
			[ Iotal veh/h	HV ] %	[ lotal   veh/h	HV J %	v/c	sec		[ Veh. veh	Dist J m		Rate	Cycles	km/h
South	: Harr	is Road													
4	L2	All MCs	147	0.0	147	0.0	0.268	35.1	LOS C	3.8	26.9	0.80	0.74	0.80	29.2
5	T1	All MCs	317	7.3	317	7.3	*0.671	36.5	LOS C	10.0	74.2	0.93	0.81	0.93	23.9
6	R2	All MCs	35	3.0	35	3.0	0.671	42.2	LOS C	10.0	74.2	0.94	0.81	0.94	32.0
Appro	ach		499	4.9	499	4.9	0.671	36.5	LOS C	10.0	74.2	0.89	0.79	0.89	26.5
East:	Quee	ns Road													
7	L2	All MCs	94	1.1	94	1.1	0.666	30.3	LOS C	13.5	95.9	0.86	0.78	0.86	30.8
8	T1	All MCs	400	1.6	400	1.6	*0.666	27.0	LOS B	13.5	95.9	0.86	0.78	0.86	30.7
9	R2	All MCs	24	0.0	24	0.0	0.666	30.5	LOS C	13.5	95.9	0.86	0.78	0.86	26.2
Appro	ach		518	1.4	518	1.4	0.666	27.8	LOS B	13.5	95.9	0.86	0.78	0.86	30.6
North:	Harri	s Road													
10	L2	All MCs	69	3.0	69	3.0	0.548	37.5	LOS C	8.8	63.3	0.89	0.77	0.89	24.2
11	T1	All MCs	469	2.7	469	2.7	0.548	35.2	LOS C	8.8	63.3	0.89	0.77	0.89	23.8
12	R2	All MCs	46	0.0	46	0.0	0.548	39.9	LOS C	7.4	52.9	0.90	0.77	0.90	23.5
Appro	ach		585	2.5	585	2.5	0.548	35.8	LOS C	8.8	63.3	0.89	0.77	0.89	23.8
West:	Quee	ns Road													
1	L2	All MCs	153	4.1	153	4.1	0.193	24.4	LOS B	3.1	22.1	0.65	0.70	0.65	27.7
2	T1	All MCs	465	2.5	465	2.5	0.559	24.7	LOS B	11.3	80.3	0.80	0.71	0.80	31.5
Appro	ach		618	2.9	618	2.9	0.559	24.6	LOS B	11.3	80.3	0.76	0.70	0.76	30.8
All Ve	hicles		2220	2.9	2220	2.9	0.671	31.0	LOS C	13.5	95.9	0.85	0.76	0.85	28.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

\* Critical Movement (Signal Timing)

Pedestrian M	Noveme	ent Perf	ormand	e:							
Mov	Input	Dem.	Aver.	Level of	AVERAGI	E BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID Crossing	Vol.	Flow	Delay	Service	QL [Ped	IEUE Dist ]	Que	Stop Rate	Time	Dist.	Speed
	ped/h	ped/h	sec		ped	m			sec	m	m/sec
South: Harris	Road										

P2 Full	50	53	49.3	LOS E	0.2	0.2	0.95	0.95	214.5	214.8	1.00
East: Queens	Road										
P3 Full	50	53	49.3	LOS E	0.2	0.2	0.95	0.95	211.8	211.3	1.00
North: Harris F	Road										
P4 Full	50	53	49.3	LOS E	0.2	0.2	0.95	0.95	214.7	215.0	1.00
West: Queens	Road										
P1 Full	50	53	49.3	LOS E	0.2	0.2	0.95	0.95	213.0	212.9	1.00
All Pedestrians	200	211	49.3	LOS E	0.2	0.2	0.95	0.95	213.5	213.5	1.00

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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# Site: 0433 [Queens Rd/ Harris Rd\_AM\_Existing - Scenario 2 (Site Folder: Left turn ban)]

Output produced by SIDRA INTERSECTION Version: 9.0.1.9664

Reprocess the Site in this Version to see the selected Movement Class results. All results may be affected by reprocessing compared with Version 9.0 results.

New Site

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 110 seconds (Site User-Given Cycle Time)

Vehic	le Mo	ovement	t Perfo	rma	nce										
Mov	Turn	Mov	Den	nand	Ar	rival	Deg.	Aver.	Level of	Aver.	Back Of	Prop.	Eff.	Aver.	Aver.
ID		Class	F	lows	FI	ows	Satn	Delay	Service	Q	lene	Que	Stop	No. of	Speed
			[ Total	HV ]	[ Total	HV]	vic	200		[Veh.	Dist ]		Rate	Cycles	km/h
South	Harr	is Road	VCH/IT	70	VCII/II	70	V/C	300	_	Ven		_	_	_	N11/11
4	L2	All MCs	147	0.0	147	0.0	0.266	35.1	LOS C	3.8	26.6	0.80	0.74	0.80	29.2
5	T1	All MCs	313	6.7	313	6.7	*0.664	36.5	LOS C	9.9	73.1	0.93	0.81	0.93	23.9
6	R2	All MCs	35	3.0	35	3.0	0.664	42.1	LOS C	9.9	73.1	0.93	0.81	0.93	32.1
Appro	ach		495	4.5	495	4.5	0.664	36.4	LOS C	9.9	73.1	0.89	0.79	0.89	26.5
East:	Queer	ns Road													
7	L2	All MCs	94	1.1	94	1.1	0.673	30.5	LOS C	13.7	97.4	0.86	0.78	0.86	30.8
8	T1	All MCs	404	2.1	404	2.1	<b>*</b> 0.673	27.1	LOS B	13.7	97.4	0.86	0.78	0.86	30.6
9	R2	All MCs	24	0.0	24	0.0	0.673	30.6	LOS C	13.7	97.4	0.86	0.78	0.86	26.2
Appro	ach		522	1.8	522	1.8	0.673	27.9	LOS B	13.7	97.4	0.86	0.78	0.86	30.5
North:	Harri	s Road													
10	L2	All MCs	69	3.0	69	3.0	0.547	37.5	LOS C	8.8	63.3	0.89	0.77	0.89	24.2
11	T1	All MCs	469	2.7	469	2.7	0.547	35.2	LOS C	8.8	63.3	0.89	0.77	0.89	23.8
12	R2	All MCs	46	0.0	46	0.0	0.547	39.9	LOS C	7.4	52.9	0.90	0.77	0.90	23.5
Appro	ach		585	2.5	585	2.5	0.547	35.8	LOS C	8.8	63.3	0.89	0.77	0.89	23.8
West:	Quee	ns Road													
1	L2	All MCs	153	4.1	153	4.1	0.193	24.4	LOS B	3.1	22.1	0.65	0.70	0.65	27.7
2	T1	All MCs	465	2.5	465	2.5	0.559	24.7	LOS B	11.3	80.3	0.80	0.71	0.80	31.5
Appro	ach		618	2.9	618	2.9	0.559	24.6	LOS B	11.3	80.3	0.76	0.70	0.76	30.8
All Ve	nicles		2220	2.9	2220	2.9	0.673	31.0	LOS C	13.7	97.4	0.85	0.76	0.85	28.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

\* Critical Movement (Signal Timing)

Pedestrian M	Noveme	ent Perf	ormand	e:							
Mov	Input	Dem.	Aver.	Level of	AVERAGI	E BACK OF	Prop.	Eff.	Travel	Travel	Aver.
ID Crossing	Vol.	Flow	Delay	Service	QL [Ped	IEUE Dist ]	Que	Stop Rate	Time	Dist.	Speed
	ped/h	ped/h	sec		ped	m			sec	m	m/sec
South: Harris	Road										

P2 Full	50	53	49.3	LOS E	0.2	0.2	0.95	0.95	214.5	214.8	1.00
East: Queens	Road										
P3 Full	50	53	49.3	LOS E	0.2	0.2	0.95	0.95	211.8	211.3	1.00
North: Harris F	Road										
P4 Full	50	53	49.3	LOS E	0.2	0.2	0.95	0.95	214.7	215.0	1.00
West: Queens	Road										
P1 Full	50	53	49.3	LOS E	0.2	0.2	0.95	0.95	213.0	212.9	1.00
All Pedestrians	200	211	49.3	LOS E	0.2	0.2	0.95	0.95	213.5	213.5	1.00

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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#### V Site: 101 [Harris Road/ Kings Road\_AM\_Existing -Scenario 1 (Site Folder: Left turn ban)]

Output produced by SIDRA INTERSECTION Version: 9.0.1.9664

Reprocess the Site in this Version to see the selected Movement Class results. All results may be affected by reprocessing compared with Version 9.0 results.

New Site Site Category: (None) Give-Way (Two-Way)

#### Vehicle Movement Performance

Mov ID	Turn	Mov Class	Dem Fl [ Total veh/ <u>h</u>	nand lows HV ] <u>%</u>	Ar Fl [ Total veh/ <u>h</u>	rival lows HV ] %_	Deg. Satn v/ <u>c</u>	Aver. Delay se <u>c</u>	Level of Service	Aver. Qı [ Veh. veh	Back Of ueue Dist ] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/ <u>h</u>
South: Harris Road															
1	L2	All MCs	52	6.1	52	6.1	0.262	4.7	LOS A	0.0	0.0	0.00	0.06	0.00	55.8
2	T1	All MCs	438	6.0	438	6.0	0.262	0.0	LOS A	0.0	0.0	0.00	0.06	0.00	59.0
Appro	ach		489	6.0	489	6.0	0.262	0.5	NA	0.0	0.0	0.00	0.06	0.00	58.7
East: Kings Road															
4	L2	All MCs	28	0.0	28	0.0	0.050	7.7	LOS A	0.1	0.5	0.54	0.72	0.54	45.2
5	T1	All MCs	6	0.0	6	0.0	0.050	14.1	LOS A	0.1	0.5	0.54	0.72	0.54	51.3
Appro	ach		35	0.0	35	0.0	0.050	8.9	LOS A	0.1	0.5	0.54	0.72	0.54	46.8
North: Harris Road															
7	L2	All MCs	22	0.0	22	0.0	0.376	8.5	LOS A	0.6	4.3	0.25	0.11	0.30	56.3
8	T1	All MCs	537	2.7	537	2.7	0.376	0.9	LOS A	0.6	4.3	0.25	0.11	0.30	56.4
9	R2	All MCs	88	0.0	88	0.0	0.376	8.7	LOS A	0.6	4.3	0.25	0.11	0.30	55.8
Appro	ach		647	2.3	647	2.3	0.376	2.2	NA	0.6	4.3	0.25	0.11	0.30	56.2
West: Kings Road															
10	L2	All MCs	89	0.0	89	0.0	0.222	7.4	LOS A	0.3	2.2	0.58	0.77	0.58	50.1
11	T1	All MCs	18	0.0	18	0.0	0.222	15.0	LOS B	0.3	2.2	0.58	0.77	0.58	50.2
12	R2	All MCs	21	0.0	21	0.0	0.222	19.4	LOS B	0.3	2.2	0.58	0.77	0.58	43.5
Approach			128	0.0	128	0.0	0.222	10.4	LOS A	0.3	2.2	0.58	0.77	0.58	49.3
All Ve	nicles		1300	3.4	1300	3.4	0.376	2.6	NA	0.6	4.3	0.20	0.17	0.22	55.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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#### V Site: 101 [Harris Road/ Kings Road\_AM\_Existing - Scenario 2 (Site Folder: Left turn ban)]

Output produced by SIDRA INTERSECTION Version: 9.0.1.9664

Reprocess the Site in this Version to see the selected Movement Class results. All results may be affected by reprocessing compared with Version 9.0 results.

New Site Site Category: (None) Give-Way (Two-Way)

#### Vehicle Movement Performance

Mov Turn Mov		Mov	Demand		Arrival		Deg.	Aver.	Level of	Aver.	Aver. Back Of		Eff.	Aver.	Aver.
ID Class		FI	lows	FI	ows	Satn	Delay	Service	QL	leue	Que	Stop	No. of	Speed	
			lotal	ΗVΙ	[ lotal	HVJ				[Veh.	Dist J		Rate	Cycles	
			veh/h	%	veh/h	%	V/C	sec		veh	m				km/h
South	: Harr	is Road													
1	L2	All MCs	49	2.1	49	2.1	0.260	4.7	LOS A	0.0	0.0	0.00	0.06	0.00	56.2
2	T1	All MCs	438	6.0	438	6.0	0.260	0.0	LOS A	0.0	0.0	0.00	0.06	0.00	59.0
Appro	ach		487	5.6	487	5.6	0.260	0.5	NA	0.0	0.0	0.00	0.06	0.00	58.7
East: Kings Road															
4	L2	All MCs	28	0.0	28	0.0	0.050	7.7	LOS A	0.1	0.5	0.54	0.72	0.54	45.3
5	T1	All MCs	6	0.0	6	0.0	0.050	14.0	LOS A	0.1	0.5	0.54	0.72	0.54	51.3
Appro	ach		35	0.0	35	0.0	0.050	8.8	LOS A	0.1	0.5	0.54	0.72	0.54	46.8
North	: Harri	s Road													
7	L2	All MCs	22	0.0	22	0.0	0.375	8.5	LOS A	0.6	4.3	0.25	0.11	0.30	56.4
8	T1	All MCs	537	2.7	537	2.7	0.375	0.9	LOS A	0.6	4.3	0.25	0.11	0.30	56.4
9	R2	All MCs	88	0.0	88	0.0	0.375	8.7	LOS A	0.6	4.3	0.25	0.11	0.30	55.8
Appro	ach		647	2.3	647	2.3	0.375	2.2	NA	0.6	4.3	0.25	0.11	0.30	56.3
West: Kings Road															
10	L2	All MCs	89	0.0	89	0.0	0.222	7.4	LOS A	0.3	2.2	0.58	0.77	0.58	50.1
11	T1	All MCs	18	0.0	18	0.0	0.222	14.9	LOS B	0.3	2.2	0.58	0.77	0.58	50.2
12	R2	All MCs	21	0.0	21	0.0	0.222	19.3	LOS B	0.3	2.2	0.58	0.77	0.58	43.5
Approach			128	0.0	128	0.0	0.222	10.4	LOS A	0.3	2.2	0.58	0.77	0.58	49.3
All Vehicles		1298	3.2	1298	3.2	0.375	2.5	NA	0.6	4.3	0.20	0.17	0.22	55.6	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

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Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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